### 4.2 Load carrying capacity of pile:

$>$ The ultimate Load carrying capacity of pile or ultimate Load bearing resistance of pile is the maximum load which it can be carry without failure.
$>$ The pile transfer the load in two ways

1) Through the tip in compression is called end bearing or point bearing
2) By shear along the surface is called as skin friction

- All type of pile behave both end bearing and skin friction

The Load carrying capacity of pile can be determined by following method

1. Dynamic Analysis
2. Static Analysis
3. Pile load test
4. Penetration test

## 1. Dynamic Analysis:

The load carrying capacity of a driven pile can be estimated from the resistance against penetration developed during driving operation with a hammer.

## i)Engineering news formula

$$
\begin{gathered}
Q_{a}=\frac{W H}{F(S+C)} \\
Q_{a}=\text { allowable load }
\end{gathered}
$$

$\mathrm{H}=$ height of fall
$\mathrm{W}=$ weight of hammer in kg
$\mathrm{F}=$ Factor of safety=6
$\mathrm{S}=$ settlement or penetration
$\mathrm{C}=$ empirical constant $=2.5 \mathrm{~cm}$
ii) Drop hammer

$$
Q_{a}=\frac{W H}{b(S+2.5)}
$$

a)single acting hammer

$$
Q a=\frac{W H}{6(s+0.25)}
$$

## b) double acting hammer

$$
Q a=\frac{(W+a p) H}{6(s+0.25)}
$$

## iii) Hileys formula

$$
\begin{gathered}
Q_{f}=\frac{W H \eta_{\mathrm{h}} \eta_{\mathrm{b}}}{s+\frac{c}{2}} \\
Q_{f}=\text { ultimate load on pile } \\
\mathrm{C}=\text { elastic compression } \\
C=c_{1}+C_{2}+C_{3} \\
c_{1}, C_{2}, C_{3}=\text { temporary elastic constant }
\end{gathered}
$$

$\mathrm{C}_{1}$ value :

Precast=0.12-0.5
Steel=0.04-0.16
Timber=0.05-0.2

$$
C_{2}=\frac{Q_{a} L}{A E}
$$

$\mathrm{C}_{3}$ value:
$\mathrm{C}_{3}=0.1=$ average value
$\mathrm{C}_{3}=0=$ hard soil
$\mathrm{C}_{3}=0.3=$ residual soil

$$
\begin{gathered}
\eta_{b}=\frac{w+e^{2} P}{W+p}, W>e p \\
\eta={\frac{w+e^{2} P}{w+p}-\left(\frac{W-e p}{w+p}\right)}^{2}, W<e p \\
b
\end{gathered}
$$

$\mathrm{P}=$ weight of pile helmet/cap
$e=$ coefficient of restitution value
For timber pile e=0
For steel pile $\mathrm{e}=0.5$

$$
\begin{gathered}
\eta_{h}=100 \%=\text { drop hammer } \\
\eta_{h}=75-85 \%=\text { single acting hammer }
\end{gathered}
$$

$\eta_{\mathrm{h}}=100 \%=$ Diesel hammer

1. For single acting hammer

$$
\mathrm{e}=0 \text { to } 0.25
$$

2. Double acting hammer

$$
\begin{gathered}
C_{1}=1.77 \cdot \frac{Q_{u}}{A} \\
C_{2}=0.0657 \frac{Q_{u} L}{A} \\
C_{3}=3.55 \frac{Q_{u}}{A}
\end{gathered}
$$

$\mathrm{A}=$ Area of pile in $\mathrm{cm}^{2}$
$\mathrm{L}=$ length of pile in m
$\mathrm{R}=$ resistance

## Safe load on pile:

$$
Q_{s}=\frac{Q}{F}
$$

## Problems:

## Dynamic Analysis:

1. A wooden pile is being driven with a drop hammer weight 20 KN and having a free fall of 1 m .the penetration in the last blow is 5 mm . Determine the load carrying capacity of pile according to engineering news formula.

## Given data:

$$
\begin{aligned}
& \mathrm{W}=20 \mathrm{KN} \\
& \mathrm{H}=1 \mathrm{~m}=100 \mathrm{~cm} \\
& \mathrm{~S}=5 \mathrm{~mm}=5 / 10 \mathrm{~cm}=0.5 \mathrm{~cm}
\end{aligned}
$$

## To find:

$$
\mathrm{Qa}=?
$$

## Soln:

i)Engineering news formula

$$
Q_{a}=\frac{W H}{F(S+C)}
$$

W.K.T F=6,C=2.5

$$
Q_{a}=\frac{20 \times 100}{6(0.5+2.5)}=111.11 \mathrm{KN}
$$

2.A reinforced concrete pile weighing 30 KN inclusive of pile cap and a dolly driven by drop hammer of weight 40 KN and having a effective fall of 0.8 m . The average set per blow is 1.4 cm . The temporary elastic compression is 1.8 cm assume the coefficient of restitution as 0.25 and FOS of 2 . Determine the allowable load.

## Given data:

concrete pile weighing $(p)=30 \mathrm{KN}$
drop hammer of weight $(\mathrm{W})=40 \mathrm{KN}$
$\mathrm{H}=0.8 \mathrm{~m}=80 \mathrm{~cm}$
$\mathrm{S}=1.4 \mathrm{~cm}$
$\mathrm{C}=1.8 \mathrm{~cm}$
$\mathrm{e}=0.25$
FOS=2

## To find:

Allowable $\operatorname{load}(\mathrm{Q})=$ ?

## Soln:

## Hileys formula

$$
\begin{aligned}
& Q_{f}=\frac{W H \eta_{\mathrm{h}} \eta_{\mathrm{b}}}{s+\frac{c}{2}} \\
& \eta_{b}=\frac{W+e^{2} p}{W+p}, W>e p \\
& \eta=\frac{W+e^{2} P}{W+p}-(\underbrace{W-e p}_{w+p}{ }^{2}, W<e p \\
& \text { ep }=0.25 \times 30=7.5 \mathrm{KN} \\
& \mathrm{~W}=40 \mathrm{KN} \\
& \mathrm{~W}>\mathrm{ep} \text {, therefore } \\
& \eta_{b}=\frac{W+e^{2} P}{W+p} \\
& \eta_{b}=\frac{\left(40+\left(0.25^{2}\right) \times 30\right)}{30+40}=\frac{41.87}{70}=0.598 \\
& \eta_{\mathrm{h}}=100 \%=1=\text { drop hammer } \\
& Q_{f}=\quad 40 \times 80 \times 1 \times 0.598
\end{aligned}
$$

$1.4+\frac{1.8}{}$
1913.6
2
$=$

い NzRNW0!|l

$$
Q_{a}=\frac{Q_{f}}{F}=\frac{832}{2}=416 \mathrm{KN}
$$

3. A reinforced concrete pie weighing 30 kN is driven by a drop hammer weighing 40 kN and having an effective fall of 0.80 m . The average set per bow is 1.40 cm . The total temporary elastic compression is 1.80 cm , assuming the coefficient of restitution as 0.25 and factor of safety of 2 . Determine the ultimate bearing capacity and the allowable load for the pile.

## Given:

$$
\begin{aligned}
& \mathrm{P}=30 \mathrm{k} \mathrm{~W} \\
& \mathrm{~W}=40 \mathrm{~K} \mathrm{~W} \\
& \eta_{\mathrm{n}} \mathrm{H}=0.8 \mathrm{~m}=80 \mathrm{cmS}=1.4 \mathrm{~cm} \\
& \mathrm{C}=1.8 \mathrm{~cm} \\
& \eta_{\mathrm{n}}=1 \text { for drop hammer. } \\
& \mathrm{P} \times \mathrm{xe}=30 \times 0.25 \\
& \quad=7.5 \mathrm{k} \mathrm{w}
\end{aligned}
$$

$\therefore \mathrm{w}>\mathrm{pe}$
Efficiency hammer blow

$$
\begin{aligned}
\therefore \eta_{\mathrm{b}} & =\mathrm{w}+\mathrm{e} 2 \mathrm{p} /(\mathrm{w}+\mathrm{p}) \\
& =40+0.252 \times 30 /(40+30) \\
\eta_{\mathrm{b}} & =0.598
\end{aligned}
$$

Ultimate load on pile

$$
\begin{aligned}
\mathrm{Q}_{\mathrm{f}} & =\eta_{\mathrm{n}} w \mathrm{H} \eta_{\mathrm{b}} /(\mathrm{s}+\mathrm{c} / 2) \\
& =1 \times 40 \times 80 \times 0.598 /(1.4+1.8 / 2) \\
& =1914.286 /(1.4+0.9)
\end{aligned}
$$

$$
\mathrm{Q}_{\mathrm{f}}=832.3 \mathrm{k} \mathrm{~W}
$$

Allowable load $=\mathrm{Q}_{\mathrm{i}} / \mathrm{FOS}$

$$
=832.3 / 2.0
$$

$\mathrm{Q}_{\mathrm{a}}=416.2 \mathrm{k} \mathrm{W}$

