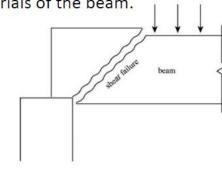
Design of Shear and torsion

Behaviour of RC members in Shear

• Shear failure occurs when the beam has **shear resistance lower than flexural strength** and the shear force exceeds the shear capacity of different materials of the beam.

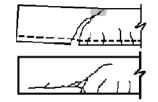






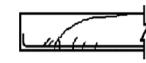
Different types of shear failure in beam

- Shear tension failure
- Shear compression failure
- Diagonal Tension Failure



Shearcompression failure

Shear-tension failure



Diagonal-tension failure

NOMINAL SHEAR STRESS

$$\tau_v = \frac{V_u}{b d}$$

 $\tau_v = nominal shear stress$

 V_u = shear force due to design load

 $b = breadth of the member, which for flanged sections shall be taken as the breadth of the web, b_w$

d = effective depth

Nominal Shear Stress in Case Beams of Varying Depth

Beams of uniform width and varying depths are commonly used in practice. Cantilever beams continuous beam with haunches at support, footings etc. fall under this category. In case of beams of varying depth the nominal shear stress is calculated by the modified equation given below.

$$\tau_v = \frac{V_u \pm \frac{M_u}{d} \tan \beta}{bd}$$

Where

 τ_v , V, b and d has same meaning as above

M = bending moment at the section

 β = angle between the top and bottom edges of the beam.

$$V_c = \tau_c. b. d$$

$$p = \frac{100 A_s}{bd}$$

Design shear strength of concrete, Tc in N/mm²

(100 As /b d)	Grade of concrete				
	M 20	M 25	M 30	M 35	M40 and above
≤ 0.15	0.28	0.29	0.29	0.29	0.30
0.25	0.36	0.36	0.37	0.37	0.38
0.50	0.48	0.49	0.50	0.50	0.51
0.75	0.56	0.57	0.59	0.59	0.60
1.00	0.62	0.64	0.66	0.67	0.68
1.25	0.67	0.70	0.71	0.73	0.74
1.50	0.72	0.74	0.76	0.78	0.79
1.75	0.75	0.78	0.80	0.82	0.84
2.00	0.79	0.82	0.84	0.86	0.88
2.25	0.81	0.85	0.88	0.90	0.92
2.50	0.82	0.88	0.91	0.93	0.95
2.75	0.82	0.90	0.94	0.96	0.98
≥ 3.00	0.82	0.92	0.96	0.99	1.01

Design of Shear Reinforcement

When τ_v exceeds τ_c given in Table 19, shear reinforcement shall be provided in any of the following forms:

- a) Vertical stirrups,
- b) Bent-up bars along with stirrups, and
- c) Inclined stirrups.

Shear reinforcement shall be provided to carry a shear equal to $V_u - \tau_c bd$ The strength of shear reinforcement V_{us} shall be calculated as below:

a) For vertical stirrups:

$$V_{\rm us} = \frac{0.87 f_{\rm y} A_{\rm sv} d}{s_{\rm v}}$$

 b) For inclined stirrups or a series of bars bent-up at different cross-sections:

$$V_{\rm us} = \frac{0.8.7 f_{\rm y} A_{\rm sv} d}{s_{\rm v}} \left(\sin\alpha + \cos\alpha\right)$$

 c) For single bar or single group of parallel bars, all bent-up at the same cross-section:

$$V_{\rm us} = 0.87 f_y A_{\rm uv} \sin \alpha$$

The minimum shear reinforcement in the form of stirrups shall be provided such that:

$$\frac{A_{sv}}{b s_v} \ge \frac{0.4}{0.87 f_v}$$

where

- A_{sv} = total cross-sectional area of stirrup legs effective in shear,
- s_v = stirrup spacing along the length of the member,
- b = breadth of the beam or breadth of the web of the web of flanged beam b_w , and
- f_{γ} = characteristic strength of the stirrup reinforcement in N/mm² which shall not be taken greater than 415 N/mm².

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Behaviour of rectangular RC beams in shear and torsion



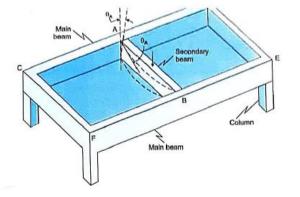
 $V_e = V_u + 1.6(T_u/b)$

where Ve = equivalent shear,

- V_u = actual shear,
- T_u = actual torsional moment,
- b = breadth of beam.
- (b) The equivalent nominal shear stress τ_{ve} is determined from:

$$\tau_{ve} = (V_e/bd)$$

, $\tau_{\rm ve}$ shall not exceed $\tau_{\rm cmax}$ given in Table 20 of IS 456



The longitudinal flexural tension reinforcement shall be determined to resist an equivalent bending moment M_{e1} as given below:

$$M_{e1} = M_{\mu} + M_{t}$$

where M_u = bending moment at the cross-section, and

$$M_t = (T_u/1.7) \{1 + (D/b)\}$$

where T_u = torsional moment,

D = overall depth of the beam, and

b = breadth of the beam.

The transverse reinforcement consisting of two legged closed loops enclosing the corner longitudinal bars shall be provided having an area of cross-section A_{sv} given below:

$$A_{sv} = \frac{T_u s_v}{b_1 d_1 (0.87 f_v)} + \frac{V_u s_v}{2.5 d_1 (0.87 f_v)}$$

However, the total transverse reinforcement shall not be less than the following:

 $A_{sv} \ge (\tau_{ve} - \tau_{e}) b s_{v} / (0.87 f_{v})$

where

 T_u = torsional moment,

 V_u = shear force,

 s_v = spacing of the stirrup reinforcement,

 b_{τ} = centre to centre distance between corner bars in the direction of the width,

 d_1 = centre to centre distance between corner bars,

