

UNIT -II  
DESIGN OF BEAMS

2.2 Design of Flanged beams

**Problem 1.** Find the flange width of the following simply supported T-beam. Effective span = 6m, C/C distance of adjacent panels = 3.0m, Breadth of the web = 350mm, Thickness of slab = 100mm.

Solutions:

Given:  $l = 6\text{m}$ ,

$$b_f = 300,$$

$$D_f = 100\text{mm}.$$

Since the beam is simply supported, the distance between the points of zero moments  $l_0 = l = 6\text{m}$

Clear span of the slab to the left or right of the beam

$$= \text{C/C distance of adjacent panels} - b_w$$

$$= 3000 - 350 = 2650\text{mm}$$

Effective width of the flange is the least of the following:

i)  $b_f = l_0 / 6 + b_w + 6D_f$

$$= 6000 / 6 + 350 + 6 \times 100 = 1950\text{mm}$$

ii)  $b_f = b_w + \text{Half of the clear distance to the adjacent beams on either side}$

$$= 350 + 2650/2 + 2650/2 = 3000\text{mm}$$

Therefore,  $b_f = 1950\text{mm}$ .

**Problem 2.** A T-beam of depth of 450 mm has a flange width of 1000 mm and depth of 120 mm. It is reinforced with 6- 20mm $\phi$  bars on tension side with a cover of 30 mm. If M-20 concrete and Fe415 steel are used. Calculate MR of beam. Take  $b_w = 300\text{mm}$ .

Solution:

Given :  $b_w = 300\text{mm}$ ,

$$b_f = 1000\text{mm},$$

$$D_f = 120\text{mm},$$

$$\text{Clear Cover} = 30\text{mm},$$

$$D = 450\text{mm} \text{ Effective cover} = 30 + 20/2 = 40\text{mm}$$

$$d = 450 - 40 = 410\text{mm}$$

$$M20, f_{ck} = 20 \text{ N/mm}^2$$

$$Fe415, f_y = 415 \text{ N/mm}^2$$

Assuming Actual Neutral Axis ( $x_u$ ) lies within the flange (i.e,  $x_u \leq D_f$ )

$$X_u/d = (0.87.f_y.A_{st}) / (0.36.f_{ck}.b.d)$$

$$= 0.87 \times 415 \times 1885 / (0.36 \times 1000 \times 20)$$

$$= 94.52 \text{ mm} < D_f (120 \text{ mm})$$

Assumption is correct

The value of  $x_{u,max} / d$  from IS:456-2000 for Fe415

$$0.48.x_{u,max} = 0.48d = 0.48 \times 410 = 196.8 \text{ mm}$$

$x_u < x_{u,max}$ , section is under reinforced, calculate the moment of resistance by the following expression

$$M_u = 0.87.f_y.A_{st}.d.[1 - ((f_y.A_{st}) / (f_{ck}.b.d))]$$

$$= 0.87 \times 415 \times 1885 \times 410 \times (1 - ((1885 \times 415) / (1000 \times 410 \times 20)))$$

$$= 252.41 \times 10^6 \text{ N-mm}$$

$$M_u = 252.41 \times 10^6 \text{ N-mm.}$$

**Problem 3. Calculate the Ultimate moment of resistance of a tee-beam having the following section properties. Use M20 and Fe 415 HYSD bars. Width of flange = 1300mm, Thickness of flange = 100mm, Width of rib = 325mm, Effective depth = 600mm, Area of steel = 4000mm<sup>2</sup>**

Solution:

Given:  $b_w = 325 \text{ mm}$ ,

$$b_f = 1300 \text{ mm},$$

$$D_f = 100 \text{ mm},$$

$$d = 600 \text{ mm},$$

$$f_{ck} = 20 \text{ N/mm}^2,$$

$$f_y = 415 \text{ N/mm}^2,$$

$$A_{st} = 4000 \text{ mm}^2.$$

Assuming Actual Neutral Axis ( $x_u$ ) lies within the flange (i.e,  $x_u \leq D_f$ )

$$X_u/d = (0.87.f_y.A_{st}) / (0.36.f_{ck}.b.d)$$

$$= 0.87 \times 415 \times 4000 / (0.36 \times 1300 \times 20)$$

$$= 154.3 \text{ mm} > D_f (100 \text{ mm})$$

Assumption is wrong, neutral axis lies below the flange.

$$D_f / d = 100 / 600 = 0.166 < 0.2$$

The value of  $x_u$  by using relation  $C_1 + C_2 = T$

$$C_1 = 0.36 \cdot f_{ck} \cdot x_u \cdot b_w = 0.36 \times 20 \times 325 \times x_u = 2340 x_u$$

$$C_2 = 0.45 \cdot f_{ck} \cdot (b_f - b_w) \cdot D_f$$

$$= 0.45 \times 20 \times 100 \times (1300 - 325)$$

$$= 877500 \text{ NT} = 0.87 \cdot f_y \cdot A_{st}$$

$$= 0.87 \times 415 \times 4000 = 1444200 \text{ N}$$

$$2340 x_u + 877500 = 1444200$$

$$x_u = 242.18 \text{ mm}$$

$$x_{u, \max} = 0.48d = 0.48 \times 600 =$$

288 mm  $x_u < x_{u, \max}$ , section is under

reinforced.  $D_f / x_u = 100 / 242.18 = 0.413$

$< 0.43$ .

Hence use equation for  $M_u$  calculation

$$M_u = 0.36 \cdot f_{ck} \cdot b_w \cdot d^2 \cdot (x_u/d) \cdot (1 - 0.42 \cdot (x_u/d)) + 0.45 \cdot f_{ck} \cdot (b_f - b_w) \cdot D_f \cdot (d - (D_f/2))$$

$$M_u = 0.36 \times (242.18/600) \times (1 - 0.42 \times (242.18/600)) \times 325 \times 600^2 \times 20 +$$

$$0.45 \times 20 \times (1300 - 325) \times 100 \times (600 - (100/2))$$

$$= 282557218 + 482625000$$

$$= 765.18 \times 10^6 \text{ N-mm}$$

$$= \mathbf{765.15 \text{ kN-m}}$$