

1. A 18mm tk plate is joint to a 16mm plate by 200mm long [effective] butt weld. Determine the strength of the joint, if

(i) A double 'V' butt joint is provided

(ii) A single 'V' butt joint is provided

Assume the grade Fe410 for the plates and for the welds which are shop welded

Given Data:-

$$L_e = 200\text{mm}$$

Grade of plate Fe=410

$$F_u = 410 \text{ N/mm}^2$$

Weld:- Shop welded

Sln:-

(i) Double 'V' butt joint:-

$$\begin{aligned} \text{Strength of weld} &= \text{Design stress of weld} \times \text{Eff. Area} \\ &= f_{wd} \times l_w \times t \end{aligned}$$

$$f_{wd} = \frac{f_{wn}}{\gamma_{mw}}$$

$$f_{wn} = \frac{f_u}{\sqrt{3}}$$

For double 'V' butt joint complete penetration of the takes place.

$$f_{wn} = 236.71 \text{ N/mm}^2$$

$$f_{wd} = \frac{236.71}{1.25}$$

$$f_{wd} = 189.368 \text{ N/mm}^2$$

$$\begin{aligned} \text{Strength of weld} &= 189.368 \times 200 \times 16 \\ &= 605.977 \text{ KN} \end{aligned}$$

(ii) Single 'V' butt joint:-

$$t = 5/8 \text{ s [Incomplete Penetration]}$$

$$= 5/8 \times 16$$

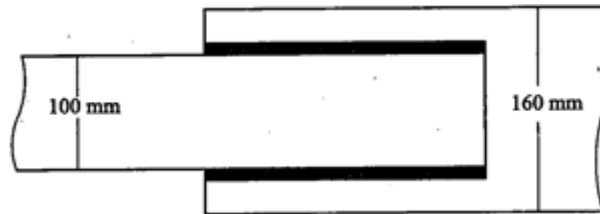
$$t = 10\text{mm}$$

$$\text{Strength of weld} = \frac{f_u / \sqrt{3}}{\gamma_{mw}} \times l_w \times t$$

$$= 189.368 \times 200 \times 10$$

$$\text{Strength of weld} = 378.74 \text{ KN}$$

2. Design a suitable longitudinal fillet weld to connect the plates as shown in fig. The pull to be transmitted is equal to the full strength of the small plate. Given the plates are 12mm tk, grade of plates is Fe410 and welding is made in the factories.



Given:-

TKS of Plate = 12mm

Grade of Plate Fe410

$f_u = 410 \text{ N/mm}^2$

$f_y = 250 \text{ N/mm}^2$

The strength of the weld is equated to the design strength of the smaller plate.

$$\text{Design strength of weld} = \frac{f_u / \sqrt{3}}{\gamma_{mw}} \times l_w \times t$$

Mini size of weld = 5mm [from Table-21 pg.No:78]

Maxi size of weld = $t_p - 1.5 = 12 - 1.5 = 10.5 \text{ mm}$

Assume the size of weld = 10mm

$$t = 0.7s$$

$$= 0.7 \times 10$$

$$t = 7 \text{ mm}$$

$$\text{Strength of smaller plate [yielding criteria]} = \frac{f_u A_g}{\gamma_o}$$

$$\text{Where, } \gamma_o = 1.1$$

Strength of smaller plate

$$\begin{aligned} \text{(yielding criteria)} &= \frac{250 \times 1200}{1.1} \\ &= 272.72 \text{ KN} \end{aligned}$$

$$\text{Strength of weld} = \frac{f_u / \sqrt{3}}{\gamma_{mw}} \times l_w \times t$$

$$272.72 \times 10^3 = \frac{410 / \sqrt{3}}{1.25} \times l_w \times 7$$

$$l_w = 205.7 \text{ mm} \approx 205 \text{ mm}$$

Provide an over lap of 105mm.

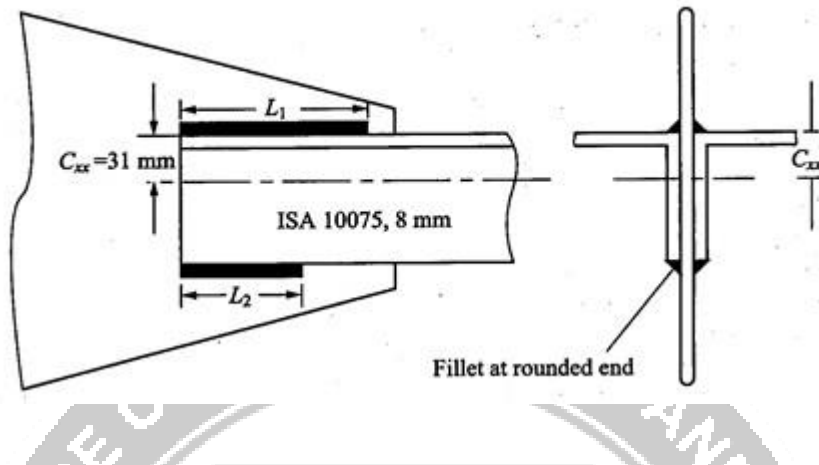
3. A tie member of a roof truss consist of 2Nos of ISA 100x75x8mm. The angles are connected to either side of a 10mm tk gusset plate and the member is subjected to a working pull of 300KN. Design the welded connection. Assume the connections are made in the shop.

Given Data:-

Working load = 300KN

2 ISA 100x75x8mm

Tks of gusset plate = 10mm



Sln:-

$$\begin{aligned}\text{Factored load} &= 1.5 \times 300 \\ &= 450 \text{ KN}\end{aligned}$$

Each ISA 100x75x8mm takes $450/2 = 225 \text{ KN}$

Min. size of weld = 3mm [From table-21 IS 800-2007]

$$\begin{aligned}\text{Max. size of weld} &= 8 - 1.5 \\ &= 6.5 \text{ mm}\end{aligned}$$

Also, max. size of weld (rounded edger) = $3/4 \times 8 = 6 \text{ mm}$

$$\begin{aligned}\text{Throat tks, } t &= 0.7 \times S \quad [\because \text{Angle of fusion} = 90^\circ] \\ &= 0.7 \times 6\end{aligned}$$

$$t = 4.2 \text{ mm}$$

Strength of weld = Design stress of weld x Eff. Area

$$\begin{aligned}&= \frac{f_u / \sqrt{3}}{\gamma_{mw}} \times l_w \times t \\ 225 \times 10^3 &= \frac{410 / \sqrt{3}}{1.25} \times l_w \times 4.2 \\ \therefore l_w &= 282.89 \text{ mm} \approx 283 \text{ mm}\end{aligned}$$

Since the C.G of angle section does not lie at the centre of the connected leg, the weld length at top & bottom need to be such that the C.G of weld.

C.G of angle ISA 100x75x8 = 31mm from the outstanding leg.

To find C.G of weld:-

Let L_1 = length of weld @ top

L_2 = length of weld @ bottom

\therefore For the C.G of the weld to lie at 31mm from the outstanding leg

$$\therefore L_1 \times 31 = L_2 (100 - 31)$$

$$L_1 = 2.23 L_2$$

$$L_1 \times L_2 = 283$$

$$2.23 L_2 + L_2 = 283$$

$$3.23 L_2 = 283$$

$$L_2 = 87.62 \text{ mm} \approx 90 \text{ mm}$$

$$L_1 = 195.39 \text{ mm} \approx 200 \text{ mm}$$

Provide 200mm length of weld @ the top and 90mm length of weld @ the bottom

∴ The min. over lap length is required 200mm

NOTE:-

In case the length of weld is limited, (length of overlap) end fillet weld can be provided which should also satisfy the condition C.G of weld = C.G of member.

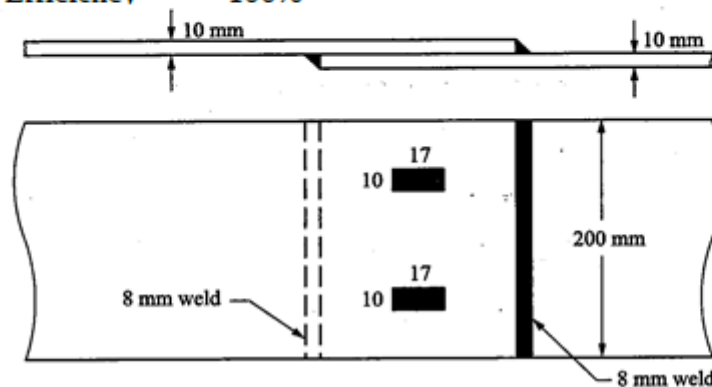
4. Design the welded connection to connect 2 plates of width 200mm & tks 10mm for 100% efficiency.

Given:-

Width of plate = 200mm

Tks of plate = 10mm

Efficiency = 100%



Sln:-

1. Strength of the solid plate:-

$$= \frac{f_y A_g}{\gamma_o}$$

$$= \frac{250 \times 200 \times 10}{1.1}$$

$$= 454.5 \text{ KN}$$

Mini. Size of weld = 3mm

Maxi. Size of weld = $10 - 1.5 = 8.5 \text{ mm}$

Assume size of weld as 8mm

Strength of the weld = Design stress of weld x Eff. Area

$$= \frac{f_u / \sqrt{3}}{\gamma_{mw}} \times l_w \times t$$

$$454.5 \times 10^3 = \frac{410/\sqrt{3}}{1.25} \times l_w \times 0.7 \times 8$$

$$l_w = 428.6 \text{ mm}$$

Total length available for weld $l = 200 + 200$

$$l = 400 \text{ mm}$$

$$\begin{aligned} \text{Eff. Length available for weld, } l_{ew} &= l - 2.5 \times 2 \\ &= 400 - 2 \times 8 \times 2 \\ l_{ew} &= 368 \text{ mm} \end{aligned}$$

To find strength of weld for 368mm:-

∴ End filled weld is provided for left = 368mm

∴ Design strength of weld for end fillet

$$\begin{aligned} &= \frac{f_u/\sqrt{3}}{\gamma_{mw}} \times l_w \times t \\ &= \frac{410/\sqrt{3}}{1.25} \times 368 \times 5.6 \end{aligned}$$

Design strength of weld for end filled = 390.2KN

Strength of weld reqd. = $454.5 - 390.2 = 64.3 \text{ KN}$

Additional weld is reqd. for this additional weld strength. Here slot weld or plug weld may be provided.

Provided plug weld, Area of plug weld reqd is,

$$\begin{aligned} \text{Area of plug weld reqd} &= \frac{\text{Additional Strength reqd.}}{\text{Design Stress of weld}} \\ &= \frac{64.3 \times 10^3}{\frac{410/\sqrt{3}}{1.25}} \\ A_w &= 339.5 \text{ mm}^2 \end{aligned}$$

∴ Provide one side of 10mm with 2 rectangular plug welds.

$$\square \text{ } l_e \text{ of one side} = 10 \text{ mm}$$

$$\therefore \text{ other side} = 2(10)x = 339.54$$

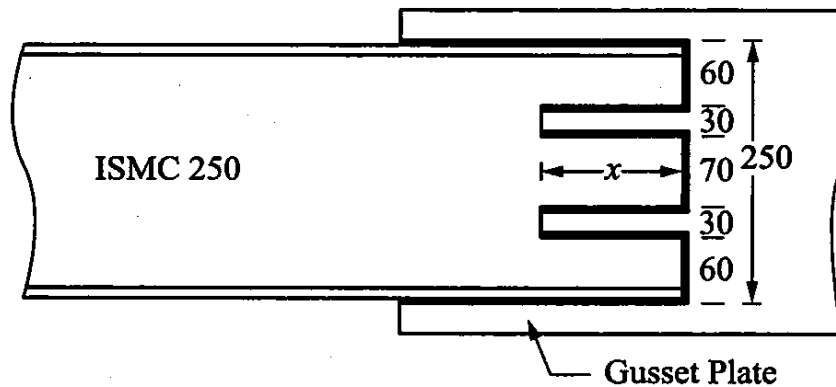
$$\therefore x = 16.97 \text{ mm}$$

The channels are connected on either side a 12mm tk gusset plate. Design the welded joint to develop full strength of the tie member. The overlap is limited to 400mm.

Given Data:-

Tks of the plate = 12mm

Tie member = IS MC 250(2Nos)



Sln:-

ISMC 250 – Properties:-

$$A = 3867\text{mm}^2$$

$$t_f = 14.1\text{mm}$$

$$t_w = 7.1\text{mm}$$

Strength of solid Plate:-

$$\begin{aligned}\text{Strength of solid plate [channel]} &= \frac{f_y A_g}{\gamma_o} \\ &= \frac{250 \times 3867}{1.1} \\ &= 878.86\text{KN}\end{aligned}$$

$$\therefore \text{Strength of weld read} = 878.86\text{KN}$$

Mini size of weld = 3mm [from table-21]

Maxi size of weld = $7.1 - 1.5 = 5.6\text{mm}$

\therefore Provide size of weld $S = 4\text{mm}$

\therefore Throat tks, $t = 0.7 \times S$

$$= 0.7 \times 4$$

$$t = 2.8\text{mm}$$

$$\begin{aligned}\text{Strength of weld} &= \frac{f_u / \sqrt{3}}{\gamma_{mw}} \times l_w \times t \\ 878.86 \times 10^3 &= \frac{410 / \sqrt{3}}{1.25} \times l_w \times 2.8\end{aligned}$$

$$l_w = 1657.48\text{mm}$$

The available length along sides & end = $400 + 250 + 400 = 1050\text{mm}$

[Since overlap is limited to 400mm]

[Either plug weld or slot weld can be provided]

Assuming 2 nos of 30mm wide to be provided along the end of the channel at equal spacing.

$$\therefore \text{Reqd length of slot} = \frac{1657 - 1050 + 2 \times 4}{4}$$

$$= 153.75\text{mm}$$

∴ Length reqd. for the slot = 153.75mm

∴ Provide 2 slots of length 154mm

